



Interdisciplinary Rockfall Workshop



2011-Innsbruck-Igls

 POLITECNICO DI MILANO



## DYNAMIC BEHAVIOUR OF ROCKFALL SHELTERS

**Francesco Calveti, Claudio di Prisco, Vittorio Moriggi**

POLITECNICO DI MILANO

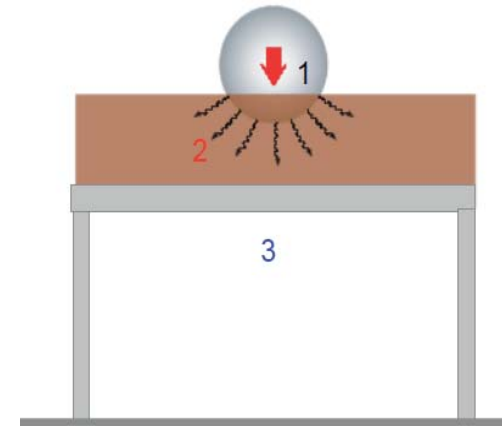


**Francesco Tondini**

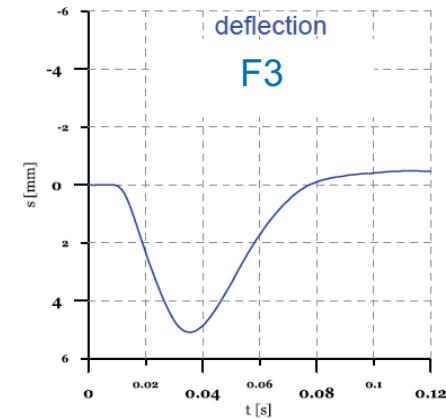
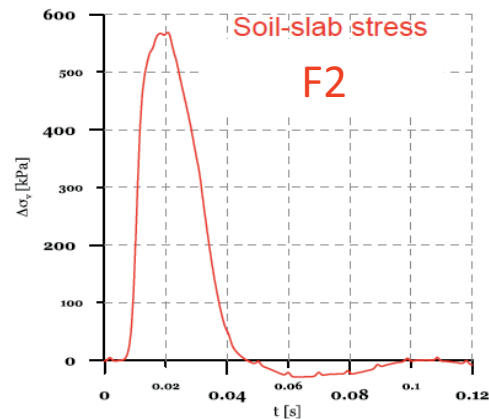
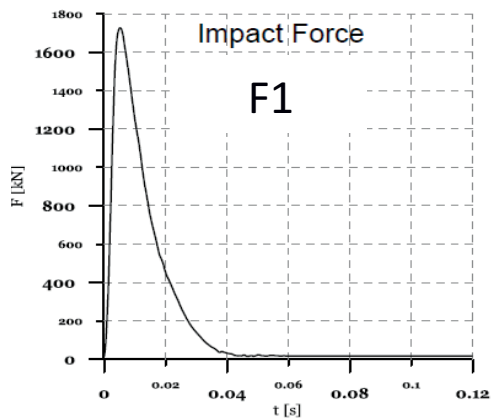




# Rockfall shelter typology: artificial tunnels covered by soil layer



Calvetti & di Prisco: An uncoupled approach for the design of rockfall protection shelters  
WSL Interdisciplinary Workshop on Rockfall Protection 2008 June 23-25, Morschach, CH



$F_1$  Impact force →  $F_2$  Actions on the shelter →  $F_3$  Structural actions

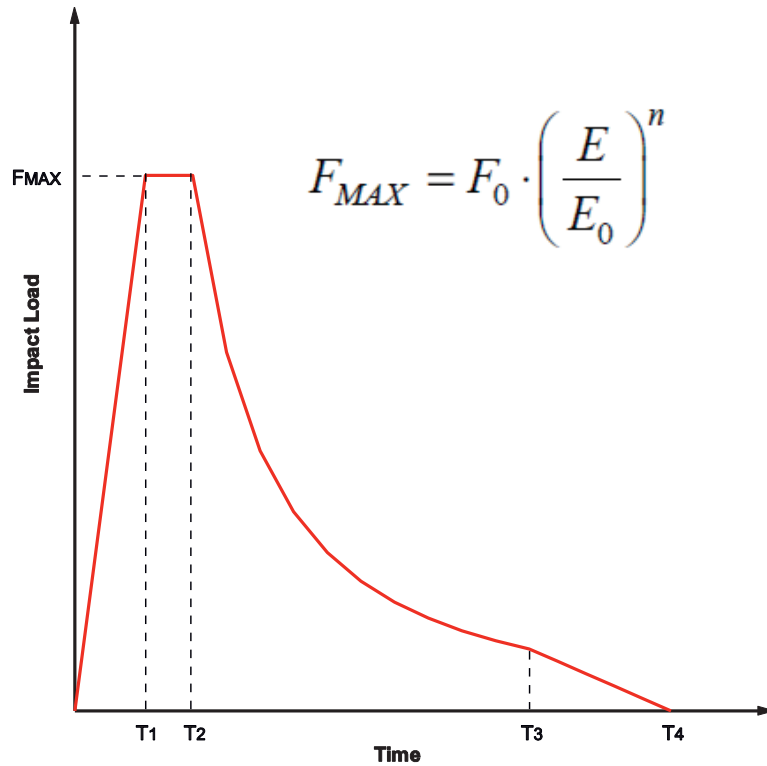
Local interaction: boulder-soil surface

Stress propagation

Structural response

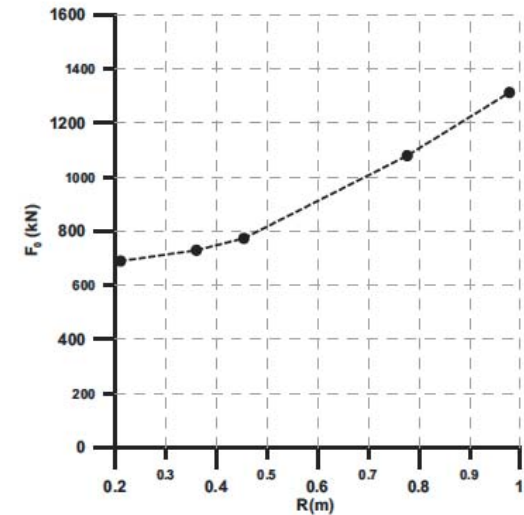
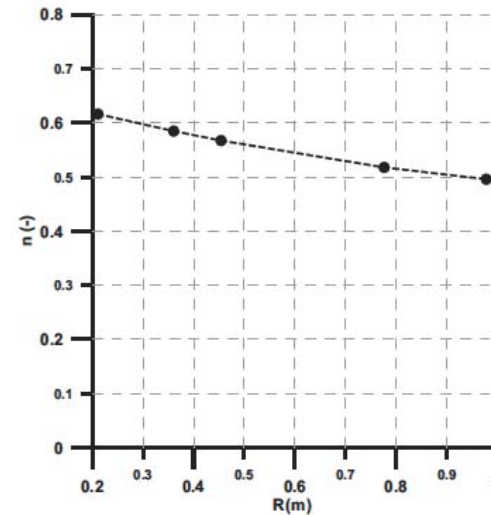


## Uncoupled design approach, STEP 1: Impact force



$$F_{MAX} = F_0 \cdot \left( \frac{E}{E_0} \right)^n$$

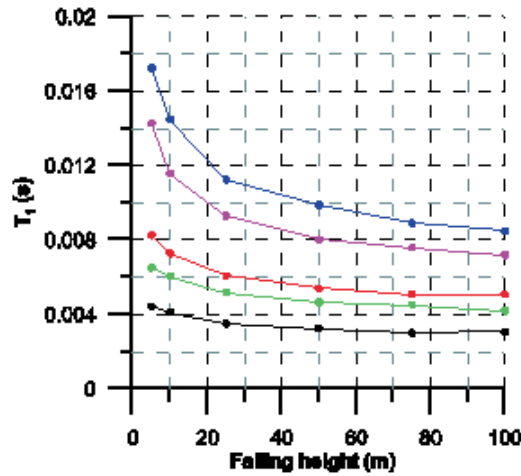
CHARTS FOR  
 $n$  and  $F_0$  as a function of block size  
(dense sand-gravel soil layer)



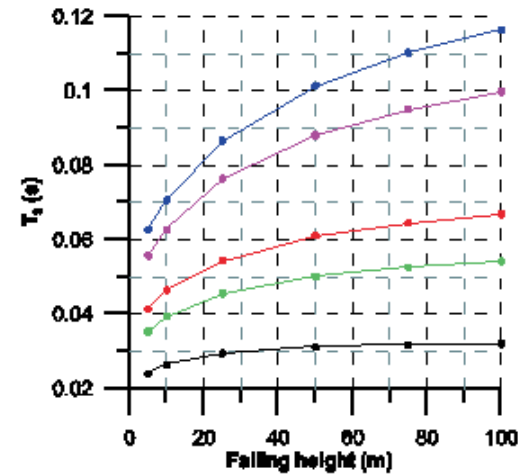


# Impact Force: design charts

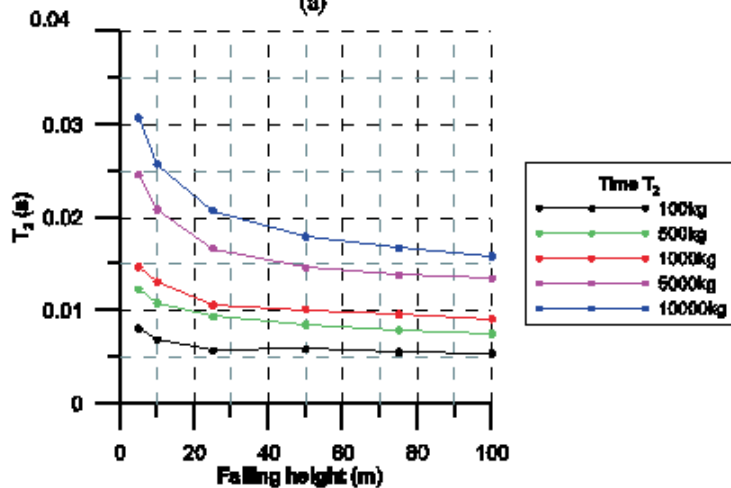
$T_{1,2,3\&4}$  as a function of falling height and block mass  
(dense sand-gravel soil layer)



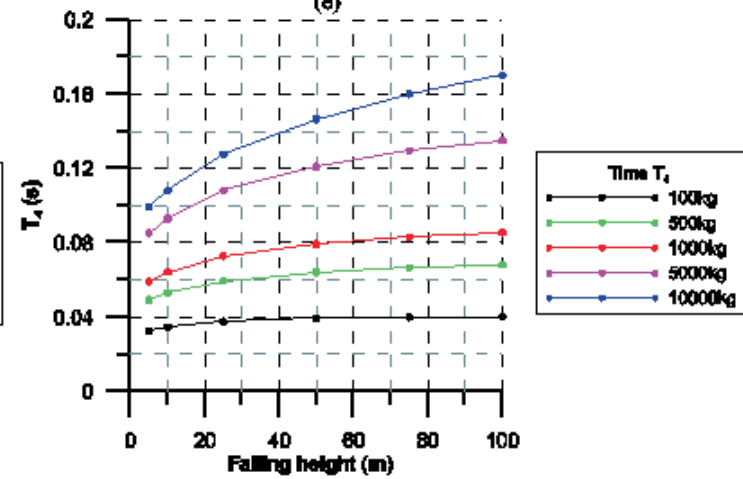
(a)



(e)



(c)

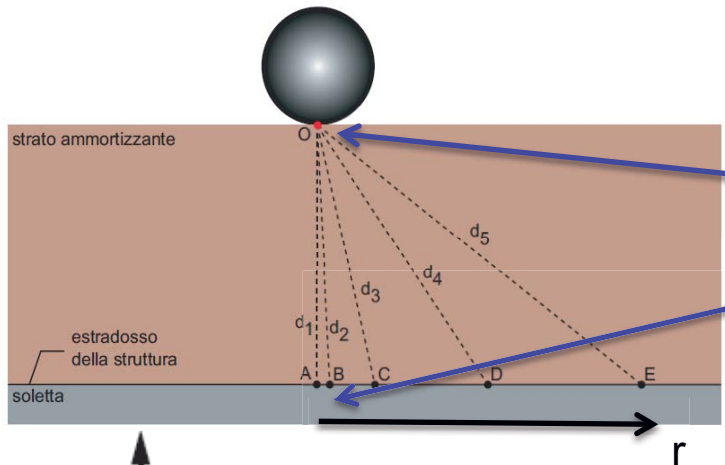


(g)



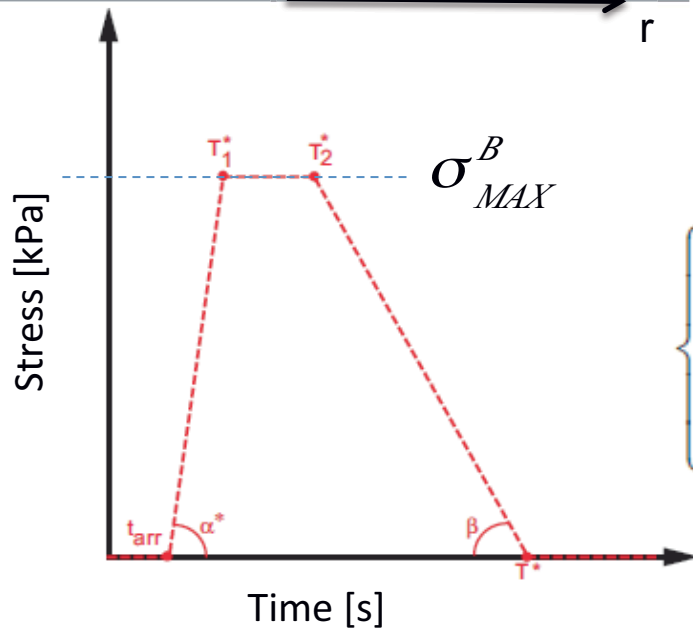
## Uncoupled design approach, STEP 2: Actions on shelter

### Impact wave propagation - dynamic stress diffusion

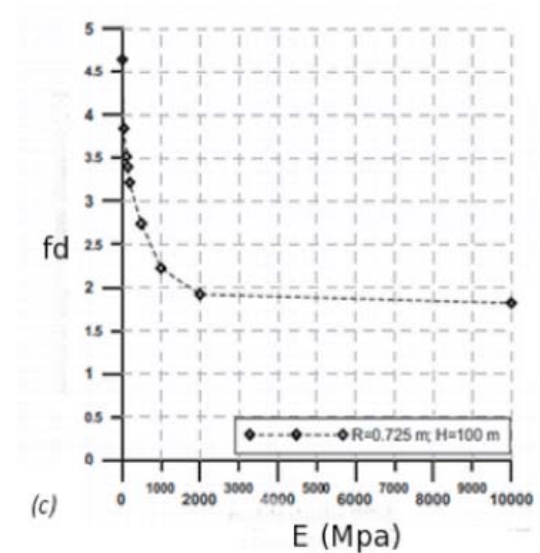


$$\sigma_{MAX}^B(r) = \sigma_{MAX}^S \cdot f_d \cdot f_g$$

Elastic solution

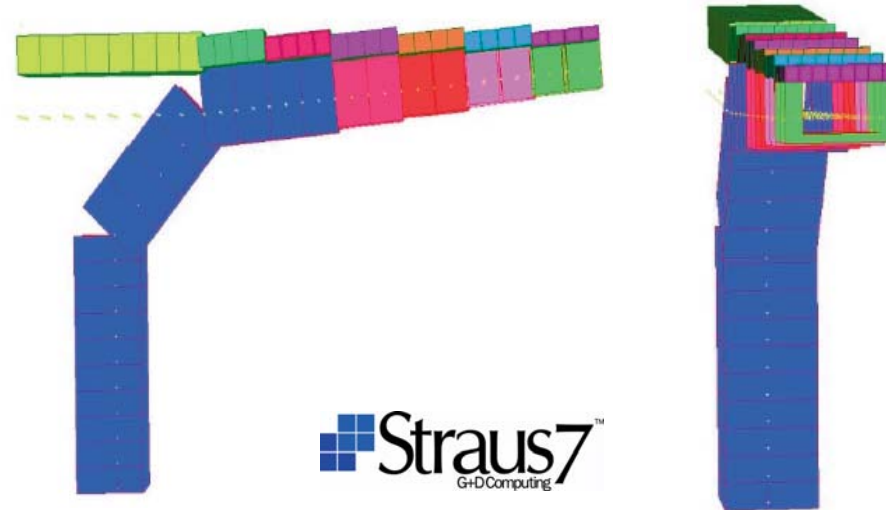
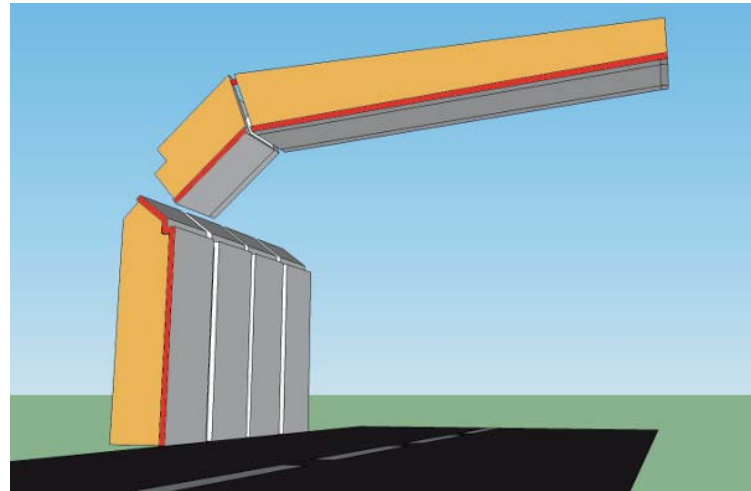
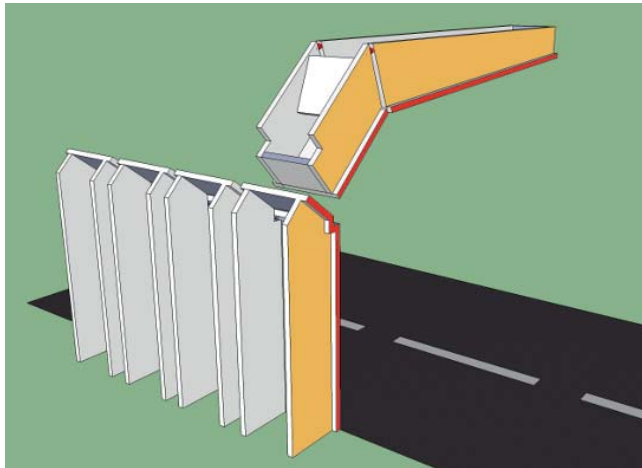


$$\begin{cases} T_1^* = t_{arr} + T_1 \\ T_2^* = 2.2 \cdot T_2 \\ T_4^* = 0.8 \cdot (t_{arr} + T_4) \end{cases}$$



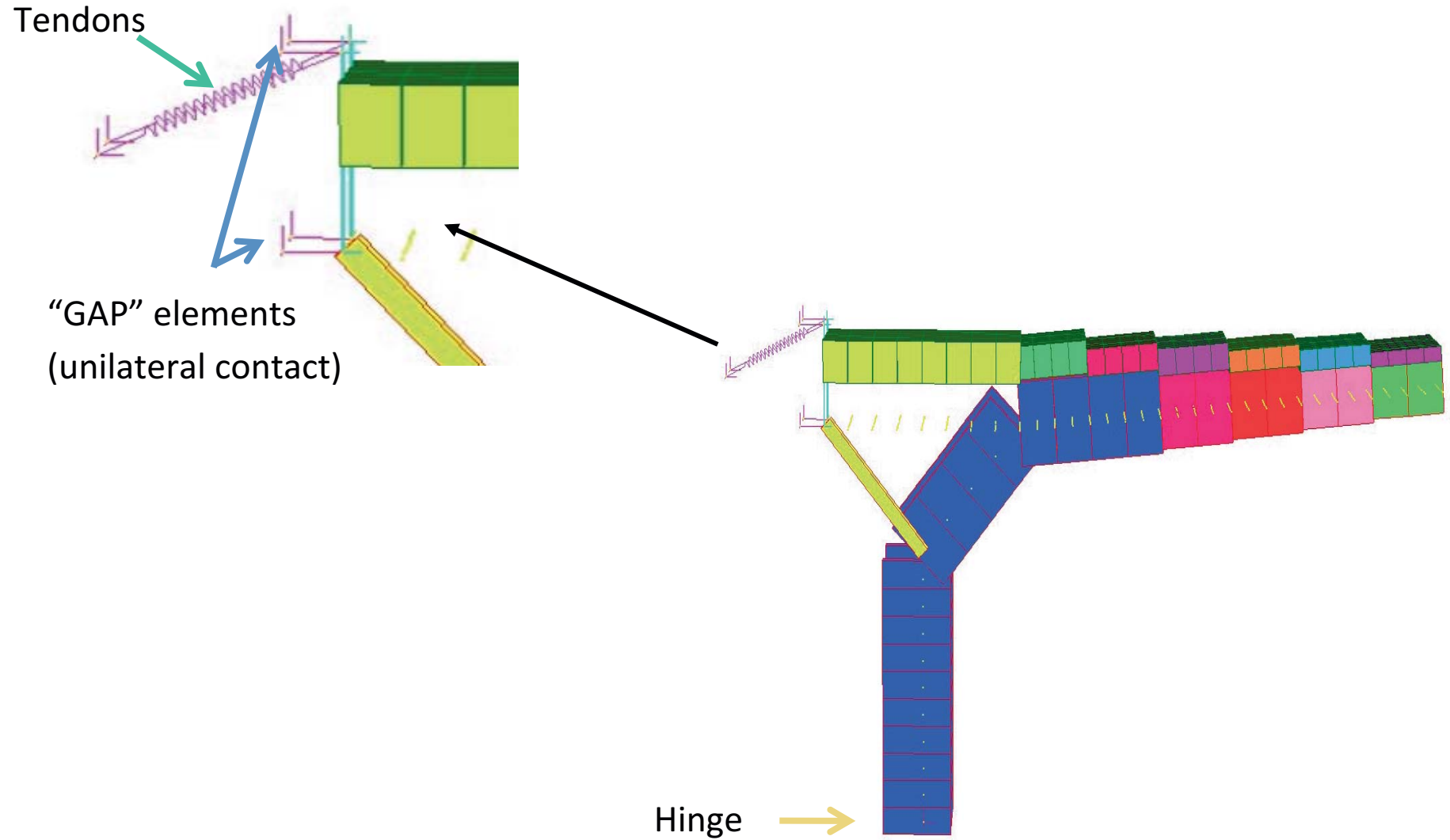


# Reference structure: cantilever shelter covered by soil cushion



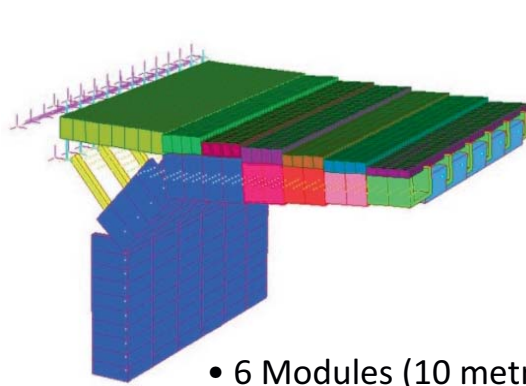


## FEM model of 1 module

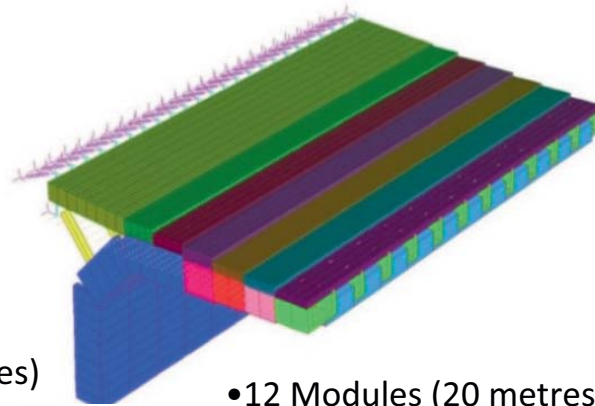




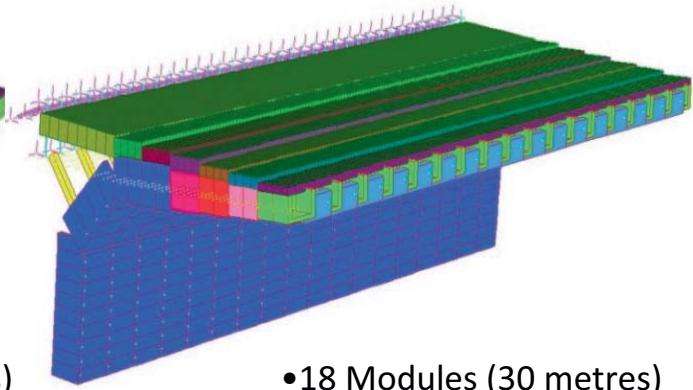
## FEM model of assembled structure



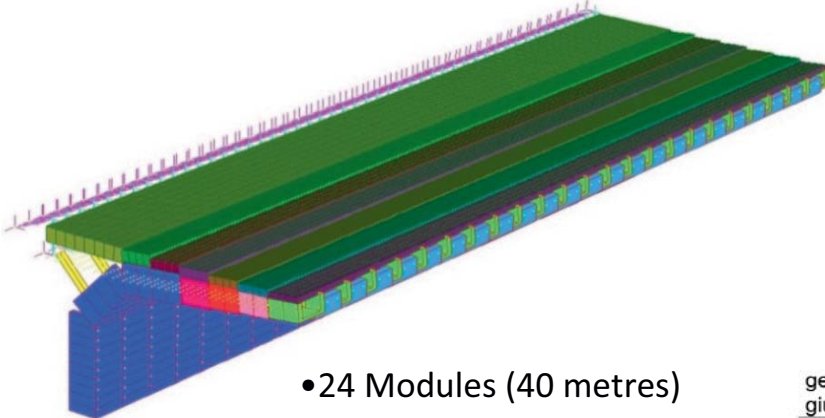
• 6 Modules (10 metres)



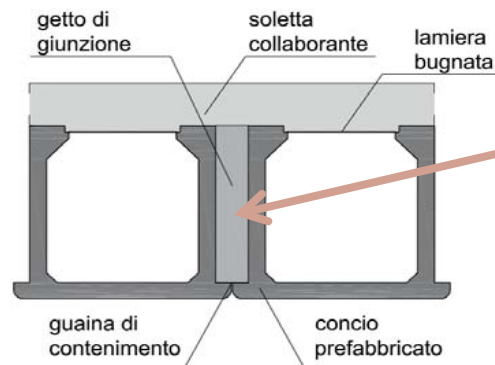
• 12 Modules (20 metres)



• 18 Modules (30 metres)



• 24 Modules (40 metres)

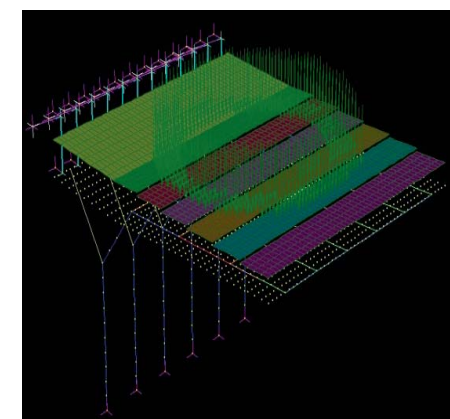
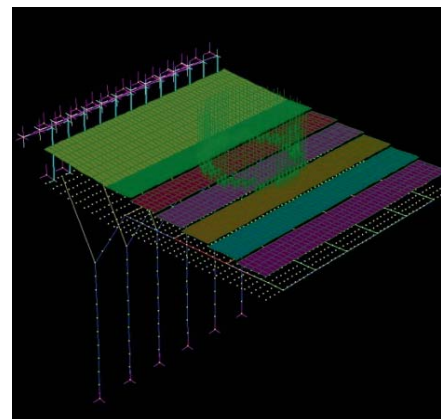
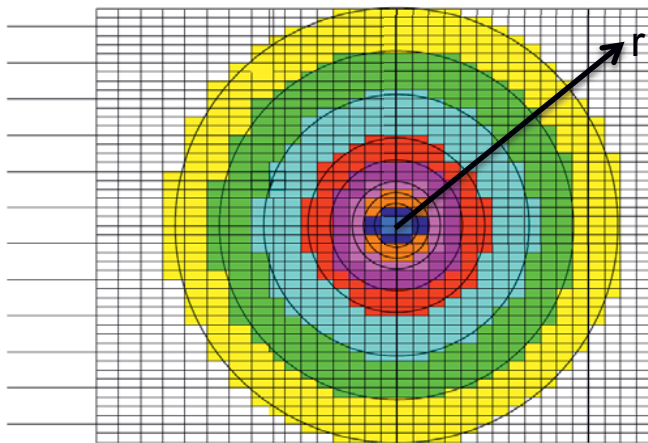
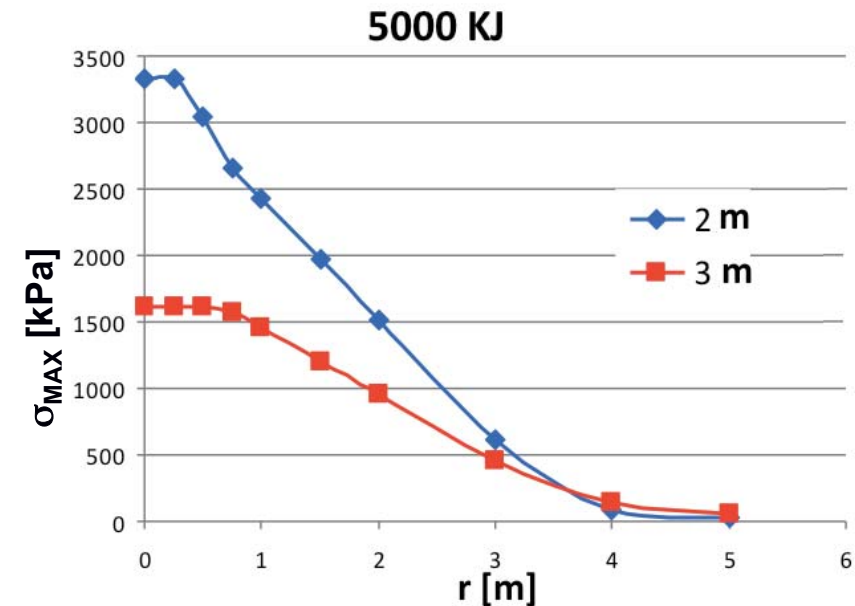
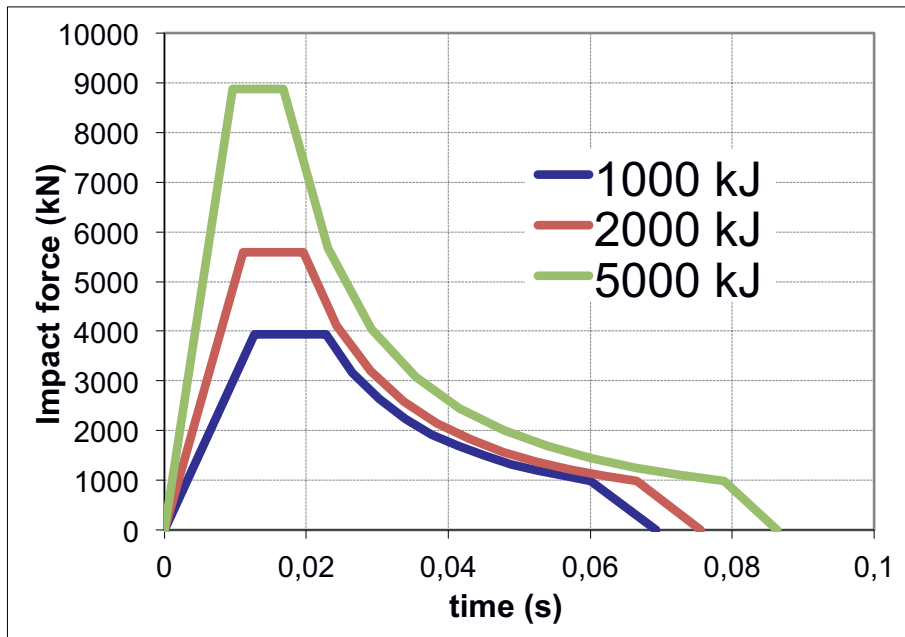


“GAP” Elements



**Reference impacts:** Block mass: 7.8 tons

Soil thickness 2-3 m (dense sand-gravel); Impact Energy :  $E = 1000-2000-5000$  kJ ( $H = 13-26-65$  m)

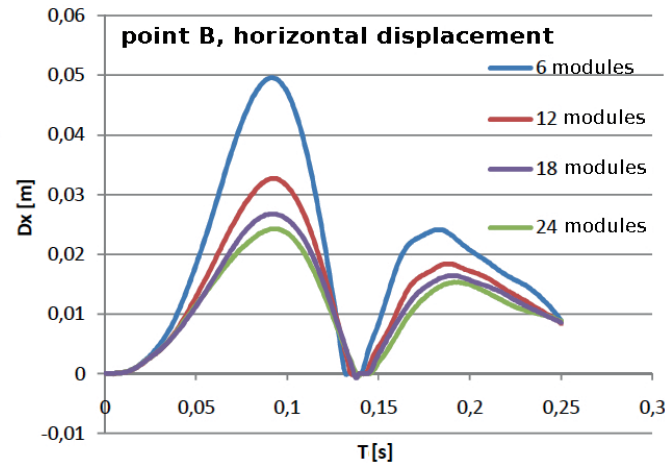
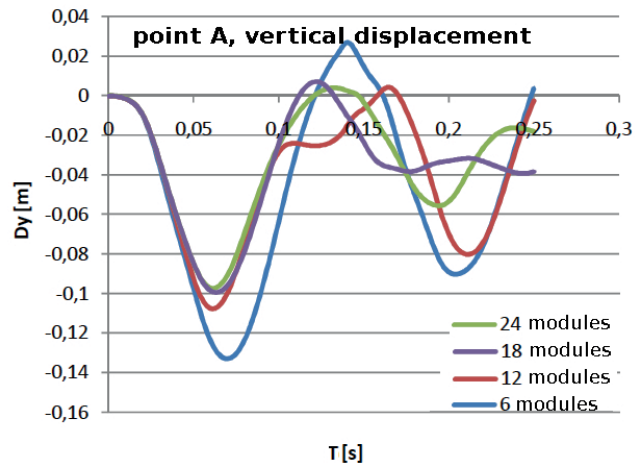
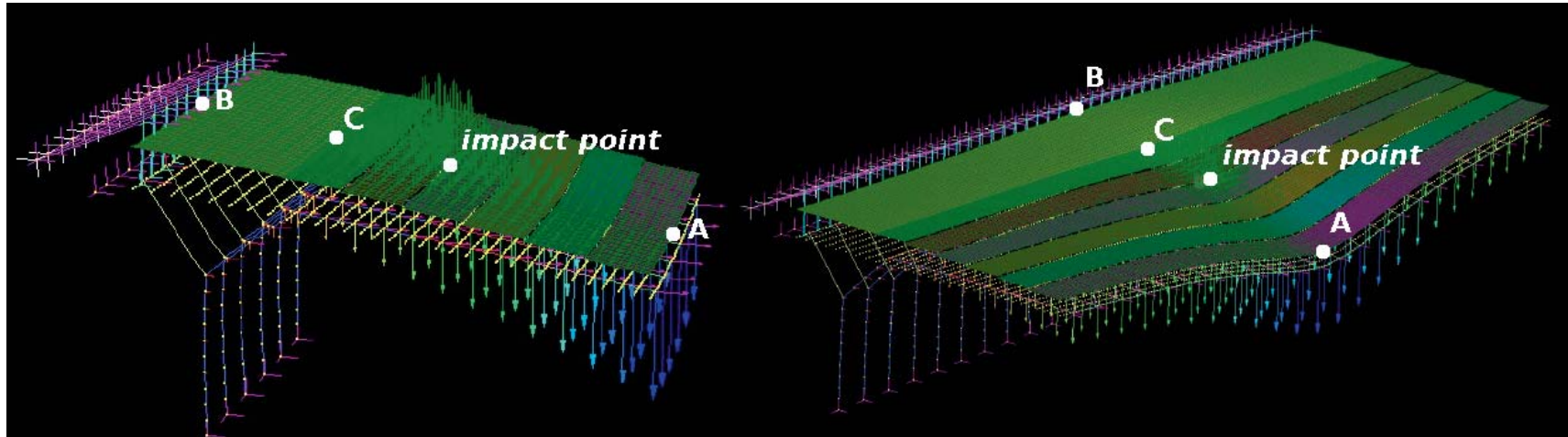




# Structural response as a function number of modules

6 modules

24 modules



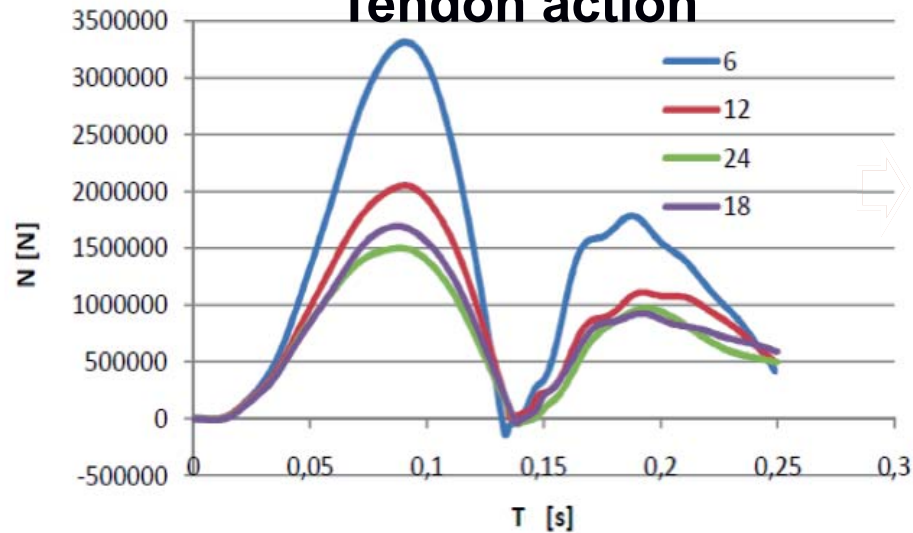
Comparison 18 – 24 modules

E	1000 kJ	2000 kJ	5000 kJ
$U_Y$	-1,8 %	-1,9 %	-0,06 %
$U_X$	-6,1 %	-7,7 %	-10,5 %



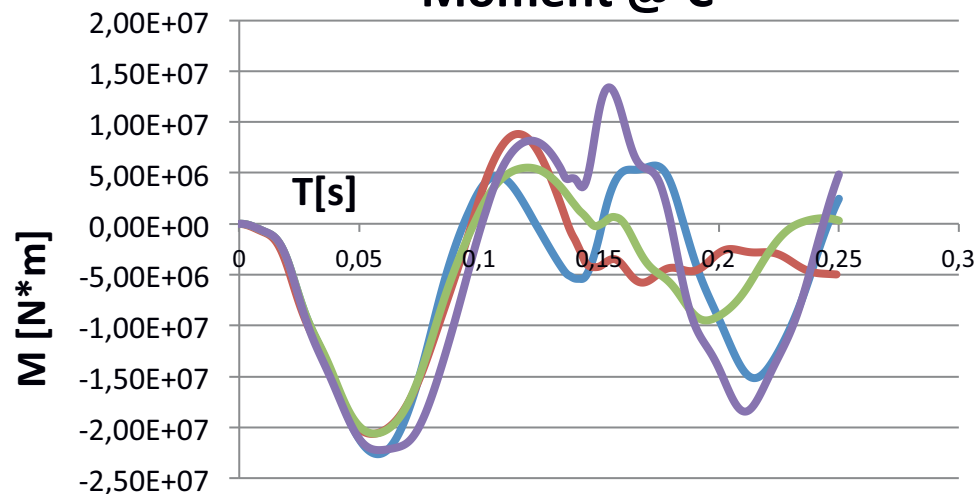
## Structural response as a function number of modules

### Tendon action



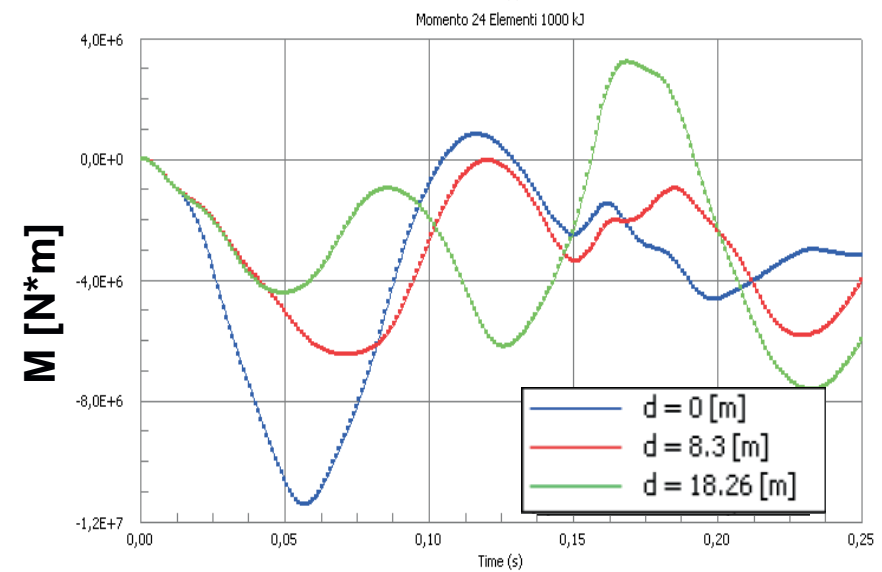
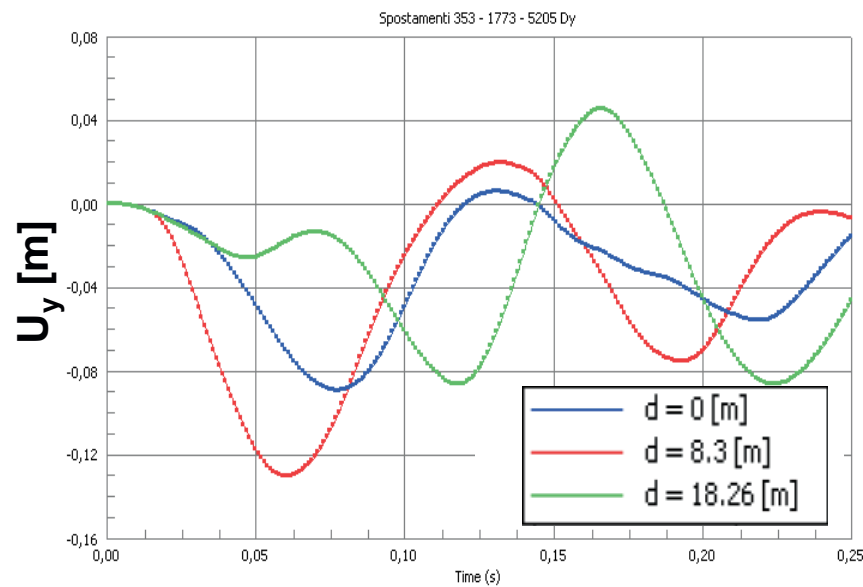
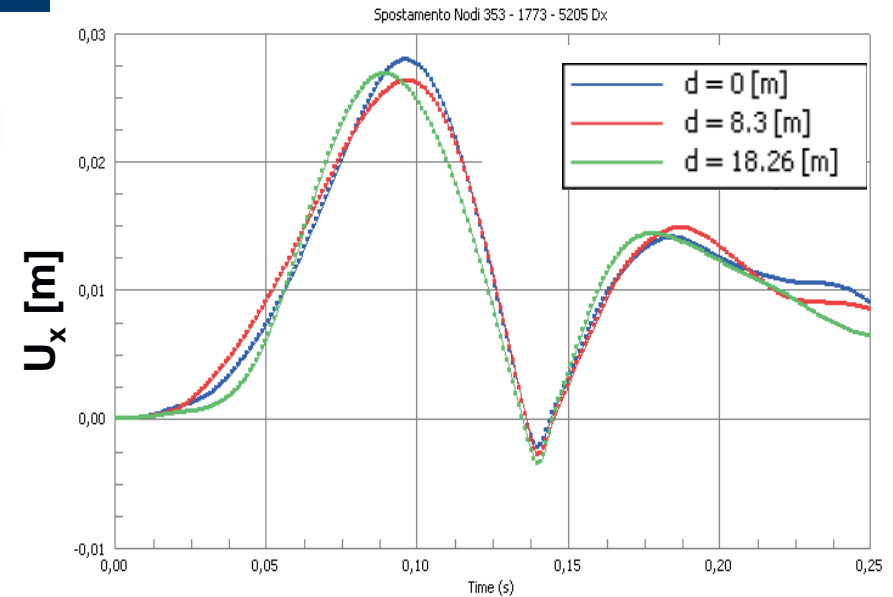
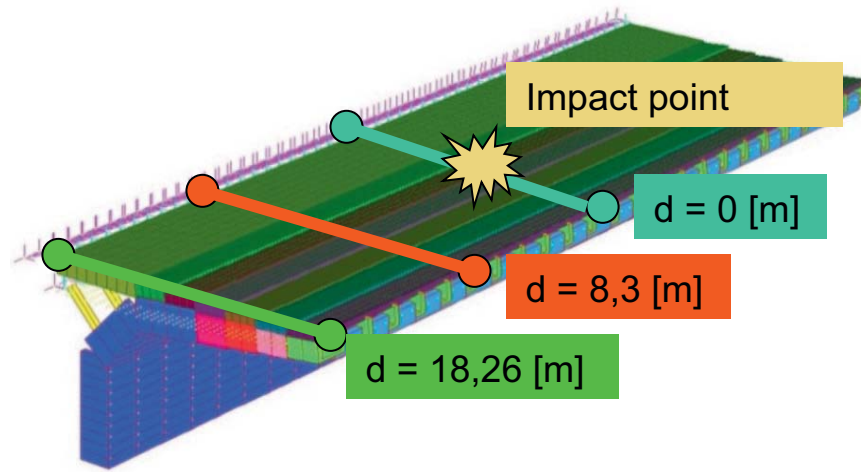
Comparison	1000 kJ	2000 kJ	5000 kJ
6-24 modules	-38,43 %	-45,58 %	-54,71 %
18-24 modules	-6,42 %	-8,44 %	-11,48 %

### Moment @ C



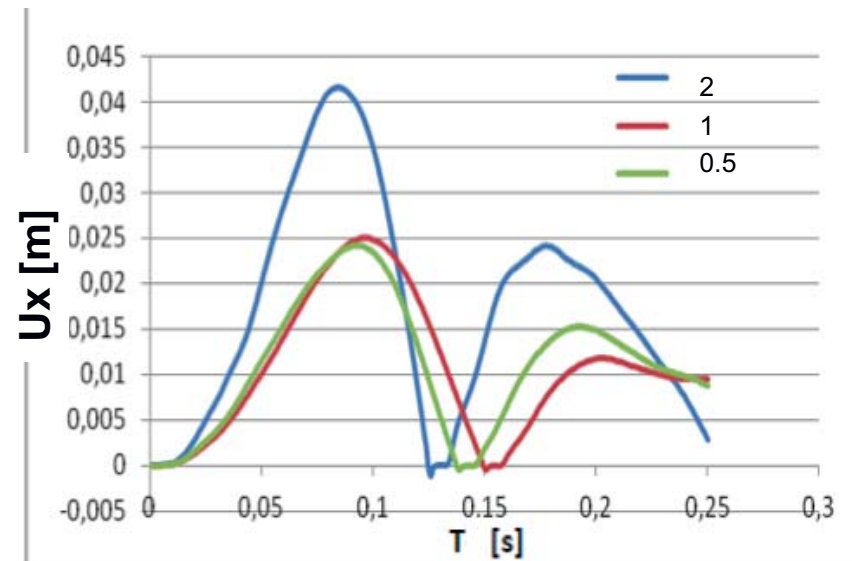
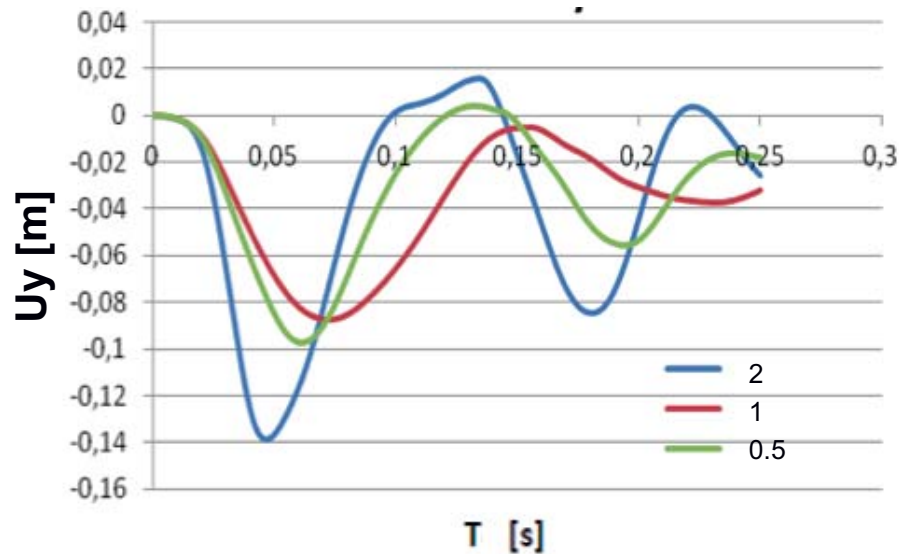
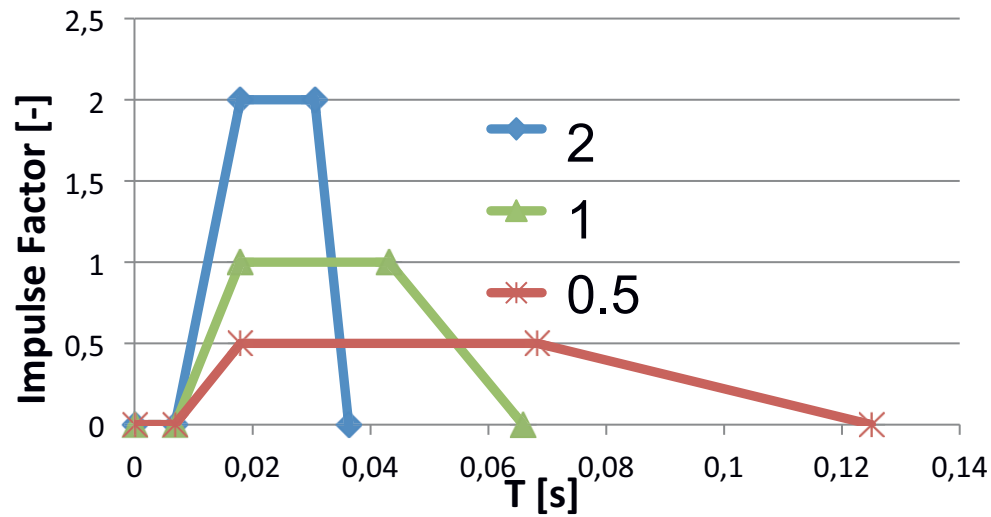


# Structural response as a function of distance from impact point





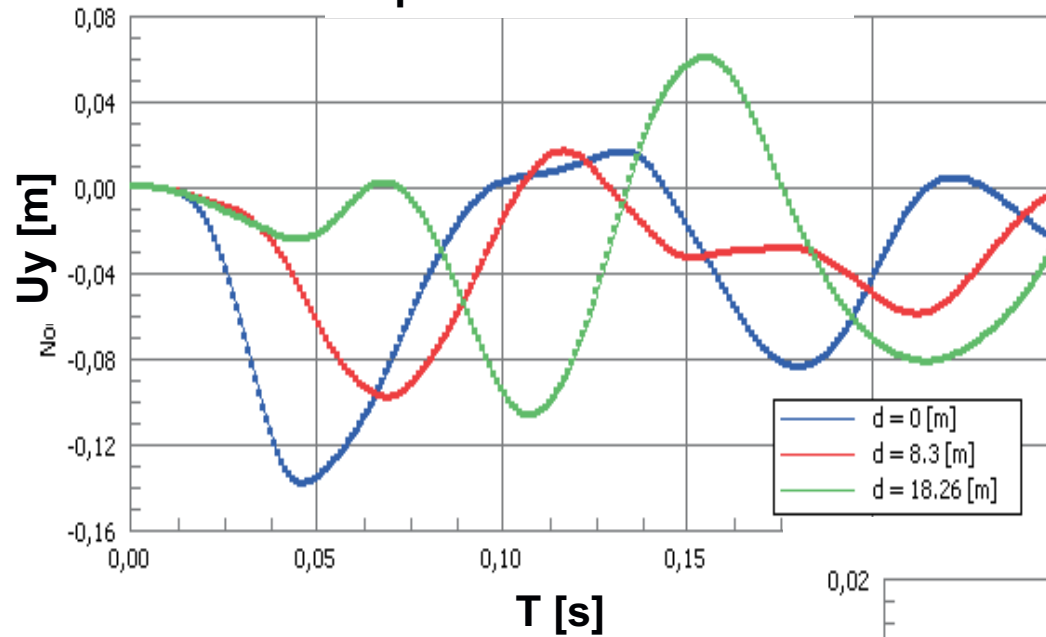
## Structural response as a function of impulse shape





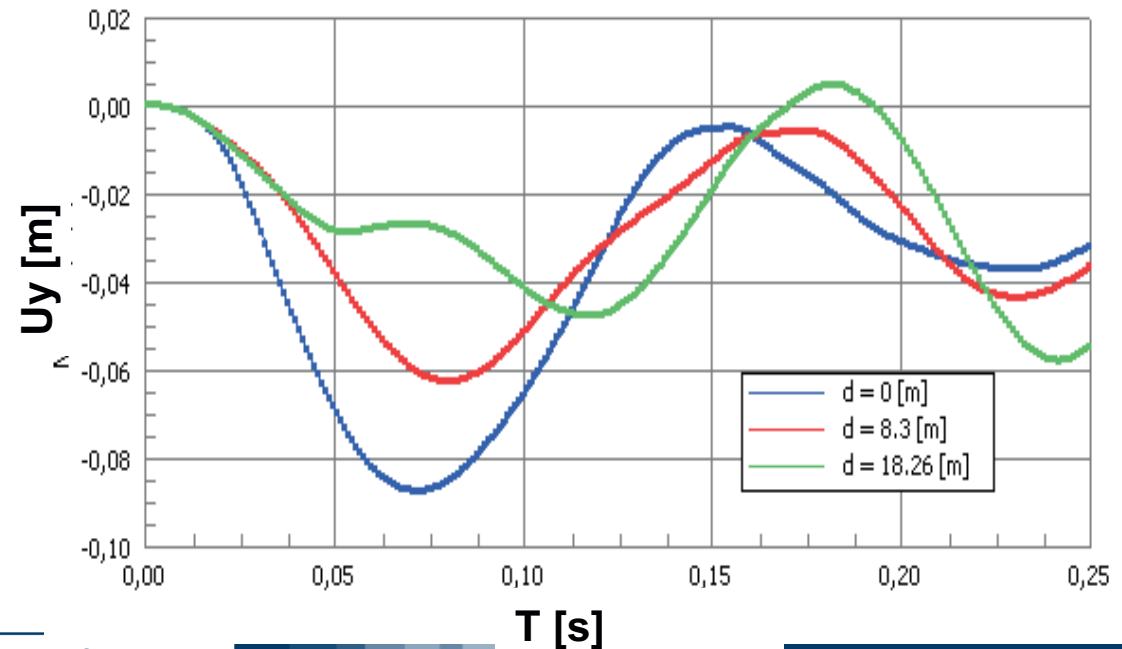
## Structural response as a function of distance from impact point

Impulse factor 2.0



Vertical displacements @ A

Impulse factor 0.5





## Structural response: dynamic amplification factors

$$R_d = \frac{u_0}{u_{st}}$$

Max. displacement under impulse load

Static displacement under peak of impulsive force

